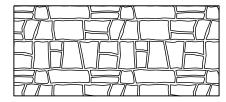


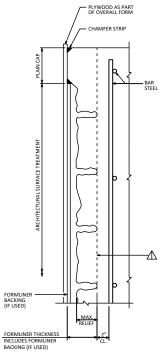
#### **BROKEN RIB**

FORMLINER THICKNESS =  $3" \pm \frac{1}{2}"$ WIDTH =  $2" \pm \frac{1}{2}"$ MAX. RELIEF =  $2" \pm \frac{1}{2}"$ 



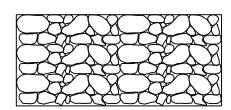
#### **RUSTIC ASHLAR**

FORMLINER THICKNESS = 3" SIZE = 8" TO 32" MAX. RELIEF = 2"



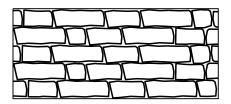
#### SECTION THRU FORMLINER

A STRUCTURAL CONCRETE CAN ONLY BE ASSUMED TO TO THIS LINE. PROVIDE ADDITIONAL STRUCTURE SIZE AS NECESSARY TO MAINTAIN MINIMUM FULL STRUCTURAL CONCRETE DIMENSIONS AS INDICATED ON THE STANDARDS.



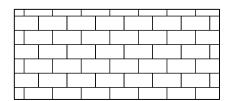
#### FIELD STONE - RANDOM

FORMLINER THICKNESS = 3½" SIZES BETWEEN 6" & 24" MAX. RELIEF = 2½"



#### RECTANGULAR CUT STONE

FORMLINER THICKNESS = 4" TO 5½" COURSE HEIGHT = ± 2" MAX. RELIEF = 3" TO 4½"



#### **RECTANGULAR BRICK**

FORMLINER THICKNESS = 2" SIZE = VARIES MAX. RELIEF = 1"

#### **RETAINING WALL NOTES**

FORMLINER COURSING ON RETAINING WALLS SHALL BE LEVEL

#### **ABUTMENT NOTES**

WARNING

FORMLINER SHOWN ON THIS STANDARD IS A NON-PARTICIPATING AMENITY (CSD).

> FORMLINER COURSING ON ABUTMENTS AND WINGS SHALL BE LEVEL. THE FORMLINER COURSING ON THE WINGS SHALL BE VERTICALLY ALIGNED WITH THE FORMLINER COURSING ON THE FRONT OF THE ABUTMENT.

THE FORMLINER PATTERN SHALL BE CONTINUOUS ACROSS CONSTRUCTION JOINTS.

WRAPAROUND/MATCH FORMLINER PATTERN AT CORNERS.

#### PIER NOTES

FORMLINER COURSING ON PIERS SHALL BE LEVEL.

THE FORMLINER COURSING ON ALL FACES OF EACH COLUMN SHALL BE VERTICALLY ALIGNED.

SPACE ADJACENT PORTIONS OF FORMLINER ON SLOPED FACE SO THAT COURSING IS ALIGNED VERTICALLY WITH COURSING ON VERTICAL FACE.

THE FORMLINER PATTERN SHALL BE CONTINUOUS ACROSS CONSTRUCTION JOINTS.

WRAPAROUND/MATCH FORMLINER PATTERN AT CORNERS.

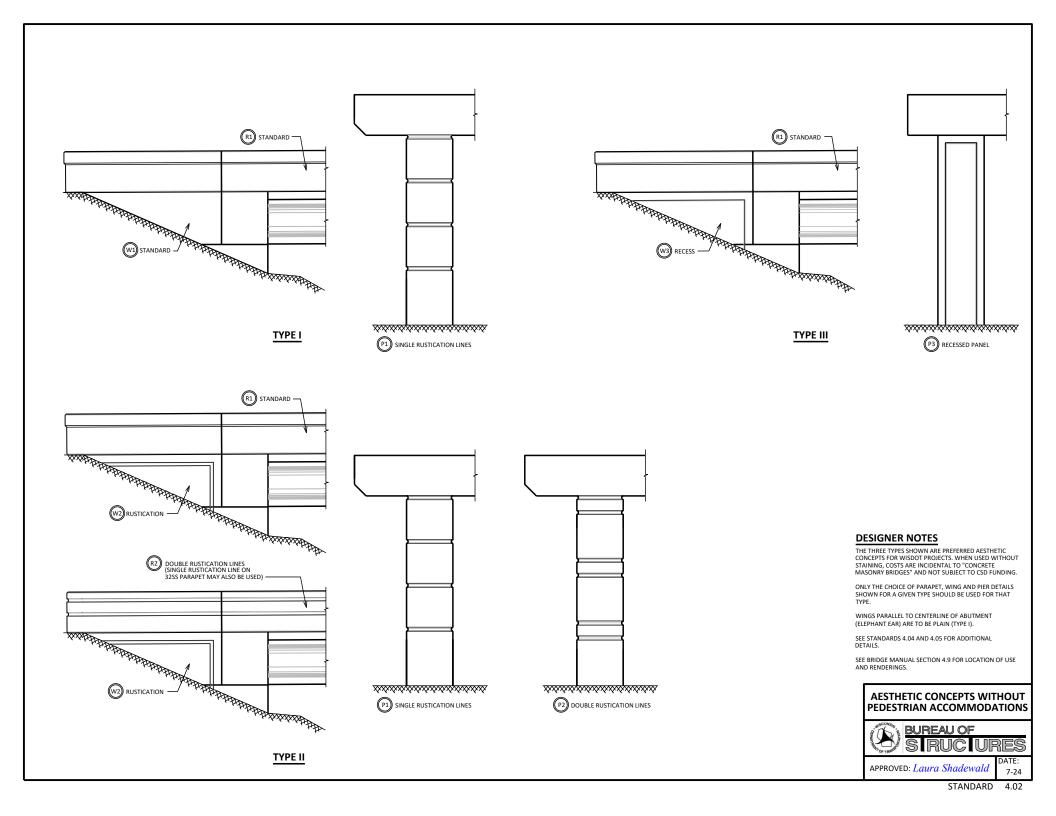
## PARAPET NOTES

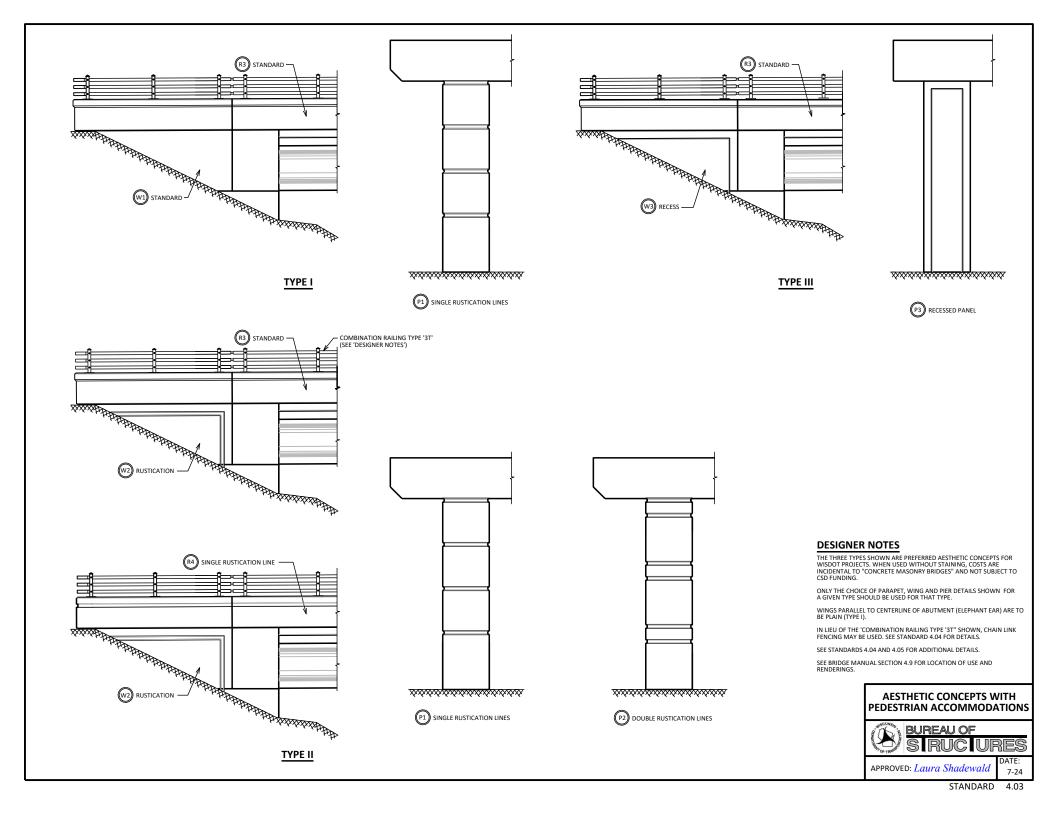
FORMLINER COURSING ON PARAPETS SHALL BE PARALLEL TO TOP OF PARAPET.

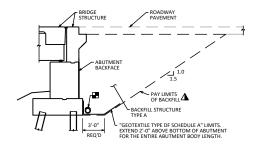
#### **FORMLINER DETAILS**



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#### - ABUTMENT BACKFACE 1.5 1.0 1.5 PAY LIMITS OF BASE AGGREGATE DENSE 11/4" PAY LIMITS A П BACKFILL STRUCTURE "GEOTEXTILE TYPE DF SCHEDULE A" LIMITS. EXTEND 2'-0" ABOVE BOTTOM OF ABUTMENT FOR THE ENTIRE ABUTMENT BODY LENGTH. REQ'D

STRUCTURAL

ROADWAY

PAVEMEN'

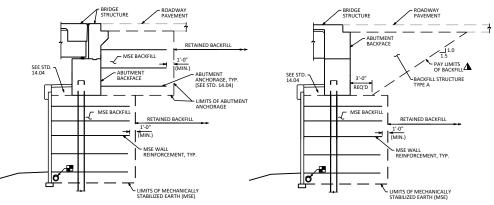
BRIDGE

STRUCTURE

#### TYPICAL SECTION THRU ABUTMENT

#### TYPICAL SECTION THRU ABUTMENT

(A3 ABUTMENT WITHOUT STRUCTURAL APPROACH)



#### **TYPICAL SECTION** THRU ABUTMENT AT MSE WALL (A3 ABUTMENT WITH ABUTMENT ANCHORAGE

THRU ABUTMENT AT MSE WALL

TYPICAL SECTION

## ABUTMENT BACKFILL DIAGRAM FOR WINGS PARALLEL TO ROADWAY

- = OUT TO OUT OF ABUTMENT, INCLUDING WINGS (FT) = AVERAGE ABUTMENT FILL HEIGHT (FT)
- $H=AVENAGE ABUTMENT FILL HEIGHT (FT) EF = EXPANSION FACTOR (1.20 FOR CY BID ITEMS AND 1.00 FOR TON BID ITEMS) <math display="block">V_{CF}=\{L(|3.0^n)(H)+(L)(0.5)(1.5H)(H)\\V_{CY}=V_{CY}(EF)/27\\V_{YOM}=V_{CY}(2.5)$



## ABUTMENT BACKFILL DIAGRAM FOR WINGS PARALLEL TO ABUTMENT

- = OUT TO OUT OF ABUTMENT BODY (FT) = AVERAGE ABUTMENT FILL HEIGHT (FT)
- = WING 1 LENGTH (FT) = WING 2 LENGTH (FT)
- = EXPANSION FACTOR (1.20 FOR CY BID ITEMS AND 1.00 FOR TON BID ITEMS)
- = (L)(3.0')(H) + (L)(0.5)(1.5H)(H) + (3.0')(0.5)(W1+W2)(H) = V<sub>CF</sub>(EF)/27 = V<sub>CY</sub>(2.0)

#### **NOTES**

THE UPPER LIMITS OF "EXCAVATION FOR STRUCTURES BRIDGES B-\_-" SHALL BE THE EXISTING GROUNDLINE.

THE BACKFILL QUANTITIES ARE BASED ON THE PAY LIMITS SHOWN ON THE PLANS AND MAY NOT REFLECT ACTUAL PLACED QUANTITIES. "BACKFILL STRUCTURE TYPE A" REQUIRED DIRECTLY BEHIND ABUTIMENTS AND ABUTIMENT WINGS FOR STEEL BACKFILL PLACED BEYOND PAY LIMITS OR EXCEEDING PLAN QUANTITIES SHALL BE INCIDENTAL TO EXCAVATION FOR STRUCTURES."

EXCAVATION BELOW THE ABUTMENT AND ABUTMENT BEDDING MATERIALS REQUIRES ENGINEER APPROVAL. GEOTEXTILE SHALL BE SET AT THE BOTTOM OF EXCAVATION AND EXTEND 2:0" ABOVE BOTTOM OF ABUTMENT. (NOTE INTENDED FOR PILE SUPPORTED ABUTMENTS. SEE DESIGNER NOTES FOR MORE INFORMATION.)

#### **DESIGNER NOTES**

THE DESIGN ENGINEER SHOULD PROVIDE ALL NECESSARY BACKFILL PAY
LIMITS AND NOTES IN ORDER TO DETERMINE QUANTITIES, FOR ABUTMENTS. PROVIDE AN ABUTMENT BACKELL DIAGRAM AS SHOWN ON THIS SHEET SEE BRIDGE MANUAL SECTIONS 6.4.2 AND 9.10 FOR ADDITIONAL INFORMATION.

SUBSURFACE DRAINAGE DETAILS AND NOTES SHOULD DIRECT DRAINAGE 

FOR ABUTMENTS WITH MSE BACKFILL BELOW THE REQUIRED "BACKFILL STRUCTURE TYPE A" WIDTH, PIPE UNDERDRAIN AND GEOTEXTILE ARE NOT REQUIRED BEHIND ABUTMENTS. PIPE UNDERDRAIN IS REQUIRED AT THE BOTTOM OF THE MSE WALL.

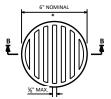
SEE STANDARD 9.02 FOR RETAINING WALL AND BOX CULVERT DETAILS.

SEE STANDARD 9.03 FOR WING FILL SECTIONS AT WING TIPS.

#### **LEGEND**

A BACKFILL PAY LIMITS. BACKFILL BEYOND BACKFILL PAY LIMITS SHALL BE INCIDENTAL TO EXCAVATION FOR STRUCTURES. LIMITS OF EXCAVATION SHALL BE DETERMINED BY THE CONTRACTOR.

PIPE UNDERDRAIN WRAPPED (6-INCH). SLOPE 0.5% MIN. TO SUITABLE DRAINAGE. ATTACH RODENT SHIELD AT ENDS OF PIPE UNDERDRAIN. (SHOW DETAIL ON PLANS)





#### **RODENT SHIELD DETAIL**

\* DIMENSIONS ARE APPROXIMATE. THE GRATE IS SIZED TO FIT INTO A PIPE COUPLING. ORIENT SO SLOTS ARE VERTICAL.

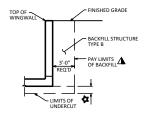
THE RODENT SHIELD, PIPE COUPLING AND SCREWS SHALL BE CONSIDERED INCIDENTAL TO THE BID ITEM "PIPE UNDERDRAIN WRAPPED 6-INCH".

THE RODENT SHIELD SHALL BE A PV. GRATE SIMILAR TO THIS DETAIL. THE GRATE IS COMMERCIALLY AVAILABLE AS A FLOOR STRANER. A PIPE COUPLING IS REQUIRED FOR THE ATTACHMENT OF THIS SHIELD TO THE EWOSED BOND OF THE PIPE UNDERFORAIN. THE SHIELD SHALL BE FASTENED TO THE PIPE COUPLING WITH TWO OR MORE NO. 10 X 1-INCH STAINLES STREEL SHEET IN TEXT ACREWS.

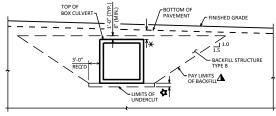
STRUCTURE BACKFILL **LIMITS AND NOTES 1** 



APPROVED: Laura Shadewald

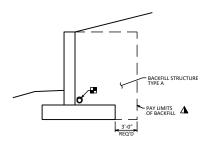


# TYPICAL SECTION THRU BOX CULVERT WINGWALL

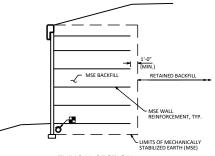


TYPICAL SECTION
THRU BOX CULVERT

(CIP BOX CULVERT WITH SHALLOW FILLS)



TYPICAL SECTION
THRU RETAINING WALL



TYPICAL SECTION
THRU MSE RETAINING WALL

#### **NOTES** (BOX CULVERTS)

THE UPPER LIMITS OF "EXCAVATION FOR STRUCTURES CULVERTS C-\_-" SHALL BE THE EXISTING GROUNDLINE.

THE BACKFILL QUANTITIES ARE BASED ON THE PAY LIMITS SHOWN ON THE PLANS AND MAY NOT REFLECT ACTUAL PLACED QUANTITIES. "BACKFILL STRUCTURE TYPE B" REQUIRED ON THE BOX COLVERT SIDES AND BEHIND A PRON WINGS FOR 3 FEET. BACKFILL PLACED BETOND PAY LIMITS OR EXCEEDING PLAN QUANTITIES SHALL BE INCIDENTAL TO EXCAVATION FOR STRUCTURES.

NOTE AND DIMENSION NOT REQUIRED. (UNDERCUT NOT REQUIRED PER GEOTECHNICAL ENGINEER OR WHEN CONSTRUCTED ON FILLS)

UNDER CUT X'-X". EXCAVATION FOR UNDER CUT TO BE INCLUDED IN EXCAVATION FOR STRUCTURES. BACKFILL WITH "BACKFILL STRUCTURE TYPE B".

UNDER CUT X'-X". EXCAVATION FOR UNDER CUT TO BE INCLUDED IN EXCAVATION FOR STRUCTURES. PLACE "GEOTEXTILE TYPE C" AND BACKFILL WITH "BREAKER RUN".

IN LEU OF USING BREAKER RUN FOR THE BOX CONSTRUCTION PLATFORM, THE CONTRACTOR MAY ELECT TO SUBSTITUTE COARSE AGGREGATE AASHTO NO. 67 OR AASHTO NO. 4 IN ACCORDANCE WITH STANDARD SPEC 310 AND 604, RESPECTIVELY, SELECT CRUSHED MATERIAL OR OTHER GRANULAR MATERIAL AS APPROVED BY THE NGINEER. THE CONTRACTOR IS RESPONSIBLE FOR BASE STABLITY WITH ANY SUBSTITUTED MATERIAL. THE REGION GEOTECHNICAL ENGINEER MAY BE CONTACTED TO DETERMINE IF "OTHER GRANULAR MATERIAL" 3 CACEPTABLE

PRECAST ELEMENTS SHALL BE PLACED ON A BEDDING OF "BACKFILL STRUCTURE TYPE B" OF 6" MINIMUM DEPTH AND AS APPROVED BY THE REGINEER, (NOTE APPLICABLE WHEN PRECAST ELEMENTS ARE ALLOWED PER STANDARD 36.02)

#### **NOTES** (RETAINING WALLS)

THE UPPER LIMITS OF "EXCAVATION FOR STRUCTURES RETAINING WALLS R-\_-\_" SHALL BE THE EXISTING GROUNDLINE.

THE BACKFILL QUANTITIES ARE BASED ON THE PAY LIMITS SHOWN ON THE PLANS AND MAY NOT REFLECT ACTUAL PLACED QUANTITIES. "BACKFILL STRUCTURE TYPE A" REQUIRED FOR THE ENTIRE WALL LENGTH. BACKFILL PLACED BEYOND PAY LIMITS OR EXCEEDING PLAN QUANTITIES SHALL BE INCIDENTAL TO EXCAVATION FOR STRUCTURES.

#### **DESIGNER NOTES**

▲ THE DESIGN ENGINEER SHOULD PROVIDE ALL NECESSARY BACKFILL PAY LIMITS AND NOTES IN ORDER TO DETERMINE QUANTITIES. SEE BRIDGE MANUAL SECTIONS 6.4.2 AND 9.10 FOR ADDITIONAL INFORMATION.

FOR CULVERTS, THE ABOVE NOTE REGARDING POTENTIAL SUBSTITUTION OF BREAKER RUN SHOULD ONLY BE INCLUDED ON THE PLANS IF ALLOWED BY THE REGION GEOTECHNICAL ENGINEER.

 $\bigstar$  6" minimum of "backfill structure type b" required between bottom of roadway surface and top of culvert.

#### LEGEND

CULVERT UNDERCUT AND BEDDING BACKFILL TO BE DETERMINED BY GEOTECHNICAL ENGINEER. (CHOOSE APPLICABLE NOTE, MODIFY AS NEEDED)

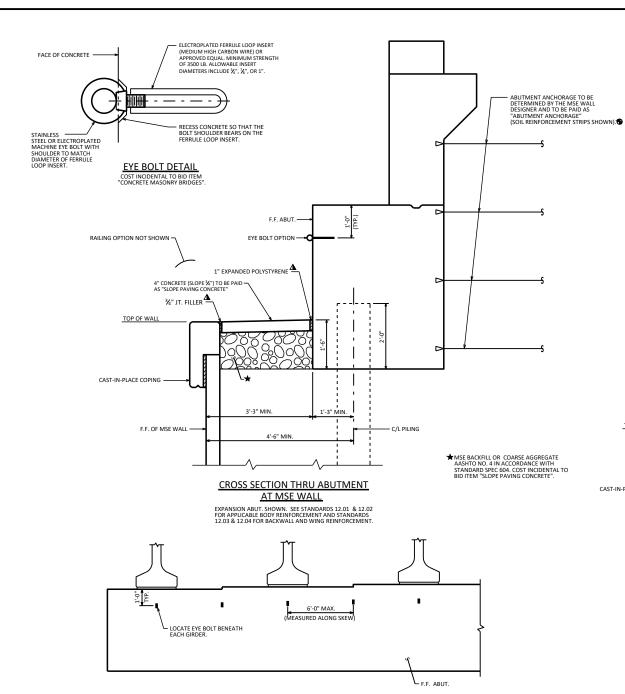
▲ BACKFILL PAY LIMITS. BACKFILL BEYOND BACKFILL PAY LIMITS SHALL BE INCIDENTAL TO EXCAVATION FOR STRUCTURES. LIMITS OF EXCAVATION SHALL BE DETERMINED BY THE CONTRACTOR.

PIPE UNDERDRAIN WRAPPED (6-INCH). SLOPE 0.5% MIN. TO SUITABLE DRAINAGE. ATTACH RODENT SHIELD AT ENDS OF PIPE UNDERDRAIN. (SHOW DETAIL ON PLANS)

STRUCTURE BACKFILL LIMITS AND NOTES 2



APPROVED: Laura Shadewald



PARTIAL ELEVATION OF F.F. ABUTMENT SHOWING EYE BOLT FALL PROTECTION OPTION **DESIGNER NOTES** 

MSE WALLS SHALL NOT BE USED FOR THE SINGULAR PURPOSE OF REDUCING SPAN LENGTH. SEE BRIDGE MANUAL SECTION 12.12 FOR ADDITIONAL INFORMATION.

FALL PROTECTION SHALL BE PROVIDED. THE OPTION PROVIDED SHOULD BE BASED ON THE PREFERENCE OF THE BRIDGE MAINTENANCE AND REGION PROJECT STAFF.

IF PIPE RAILING IS USED, SEE STD. 30.26 FOR APPLICABLE NOTES. (NOTE: STD. 30.26 IS STILL UNDER DEVELOPMENT)

"SLOPE PAVING CONCRETE" ITEMS TO BE SHOWN AS PART OF BRIDGE PLAN.

BID ITEM SHALL BE "ABUTMENT ANCHORAGE" (UNDER DEVELOPMENT).

#### **NOTES**

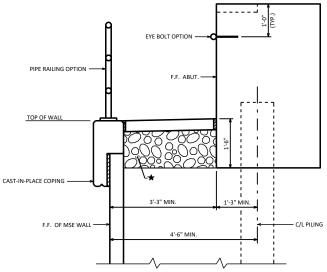
■ UNFACTORED SUPERSTRUCTURE LATERAL LOADS TRANSFERRED TO THE ABUTMENT UNFACTORED SUPERSTRUCTURE LATERAL LOADS TRANSFERRED TO THE ABUTME!
ARE TAKEN TO BE KIPS PER FOOT OF ABUTMENT LENGTH. THE VALUES ARE
TO BE USED FOR THE LIRFD DESIGN OF THE ABUTMENT ANCHORAGE BY THE MSE
MANUFACTOR (MSE SYSTEM, DEAD MAN ANCHOR, OTHER). THE FOLLOWING
AASHTO LINE LOADS SHALL BE NOTED ON PLAN:
BR = \_\_KLF
TU = \_\_KLF
WL = \_\_KLF
WL = \_\_KLF

#### FOR SEMI-EXPANSION OR FIXED TYPE A1 ABUTMENTS:

THE DESIGN OF THE WALL IN FRONT OF THE ABUTMENT SHALL INCLUDE THE HORIZONTAL EARTH LOADS AND 240 PSF LIVE LOAD SURCHARGE ACTING ON THE BACK OF THE ABUTMENT BELOW THE BEAM SEATS.

▲ SEAL ALL EXPOSED HORIZONTAL AND VERTICAL SURFACES OF FILLER AND EXPANDED POLYSTYRENE WITH NON-STAINING, GRAY NON-BITUMINOUS JOINT SEALER. (1" DEEP AND HOLD 1/8" BELOW SURFACE OF CONCRETE).

EXPANSION ABUTMENTS TO BE BACKFILLED TO A MINIMUM OF THE BEAM SEAT ELEVATION PRIOR TO PLACING GIRDERS.



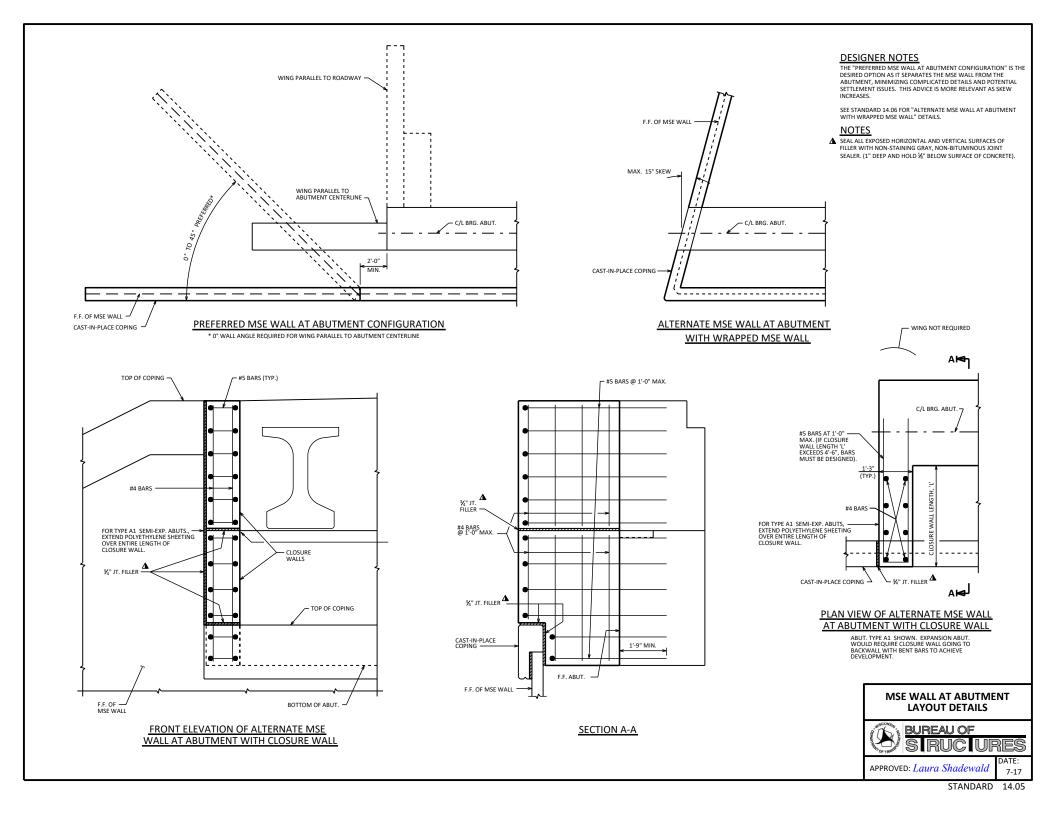
CROSS SECTION THRU ABUTMENT AT MSE WALL SHOWING BOTH EYE BOLT AND RAILING FALL PROTECTION OPTIONS

TYPE A1 SEMI-EXPANSION ABUTMENT SHOWN

**MSE WALL AT ABUTMENT** 



APPROVED: Laura Shadewald

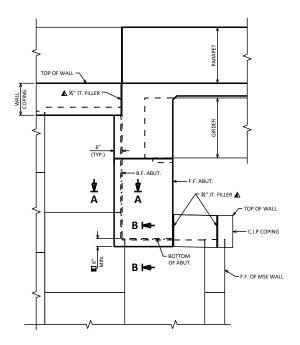


#### **DESIGNER NOTES**

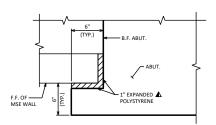
- THIS STANDARD IS FOR INFORMATIONAL PURPOSES ONLY. MODIFY DETAILS TO THE PROJECT-SPECIFIC REQUIREMENTS.
  - 6-INCH MINIMUM ASSUMES NO LONG-TERM DIFFERENTIAL SETTLEMENT BETWEEN THE ABUTMENT AND MSE WALL.
  - SEE STANDARD 14.04 FOR ADDITIONAL INFORMATION.

#### NOTES

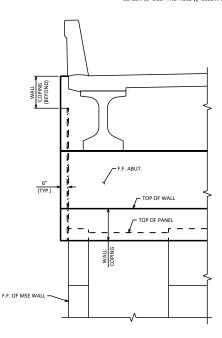
▲ SEAL ALL EXPOSED HORIZONTAL AND VERTICAL SURFACES OF FILLER WITH NON-STAINING GRAY, NON-BITUMINOUS JOINT SEALER. (1" DEEP AND HOLD 1/8" BELOW SURFACE OF CONCRETE).



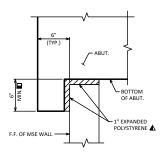
# ALTERNATE MSE WALL AT ABUTMENT WITH WRAPPED MSE WALL



**SECTION A-A** 



## FRONT ELEVATION OF ALTERNATE MSE WALL AT ABUTMENT WITH WRAPPED MSE WALL



**SECTION B-B** 

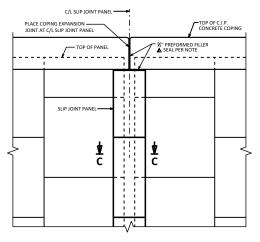
#### **DESIGNER NOTES**

SEE STANDARD 14.02 FOR C.I.P. CONCRETE COPING DETAILS.

WHEN REQUIRED FOR STAGING OR BY DESIGN, PROVIDE SLIP JOINT LOCATIONS ON THE PLANS AND PROVIDE COPING NOTES AND DETAILS TO ACCOMMODATE A SLIP JOINT. DO NOT RUN BAR STEEL THRU COPING EXPANSION JOINT.

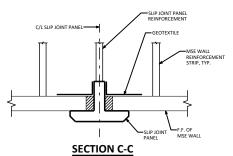
## **NOTES**

 SEAL ALL EXPOSED HORIZONTAL AND VERTICAL SURFACES OF FILLER WITH NON-STAINING GRAY, NON-BITUMINOUS JOINT SEALER. (1" DEEP AND HOLD 1/8" BELOW SURFACE OF CONCRETE).



#### C.I.P CONCRETE COPING PARTIAL ELEVATION AT SLIP JOINT

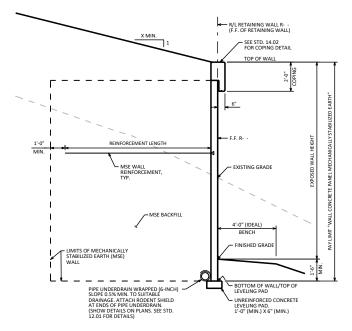
DO NOT RUN BAR STEEL THRU COPING EXPANSION JOINT (RAILING NOT SHOWN FOR CLARITY)



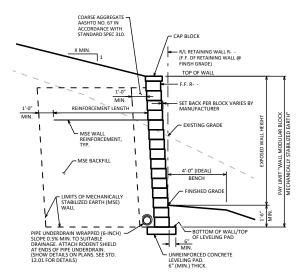
SLIP JOINT DETAIL SHOWN FOR INFORMATIONAL PURPOSES ONLY. WALL SUPPLIER TO SUBMIT JOINT DETAIL FOR ACCEPTANCE.

**MSE WALL DETAILS** 





# TYPICAL SECTION (MSE WALL WITH CONCRETE PANEL FACING)



TYPICAL SECTION

(MSE WALL WITH MODULAR BLOCK FACING)

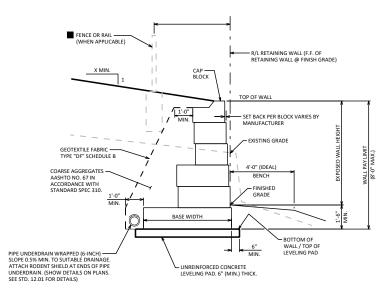
#### **DESIGNER NOTE**

SEE STANDARD 14.02 FOR ADDITIONAL INFORMATION

MSE WALL PANEL AND BLOCK FACING



APPROVED: Laura Shadewald



#### **TYPICAL SECTION**

(GRAVITY WALL WITH WET CAST BLOCK FACING)

#### **TOTAL ESTIMATED QUANTITIES**

BID ITEM NUMBER	BID ITEM	UNIT	TOTAL
612.0406	PIPE UNDERDRAIN WRAPPED 6-INCH	LF	
SPV.0165.XX	WALL MODULAR BLOCK GRAVITY (STRUCTURE)	SF	

#### **WALL EXTERNAL & OVERALL STABILITY EVALUATION**

DIMENSIONS	EVALUATED LOCATIONS		
WALL HEIGHT (FEET)			
EXPOSED WALL HEIGHT (FEET)			
MINIMUM BASE WIDTH (INCH			
WALL STATION			
BORING USED			
CAPACITY TO DEMA	AND RATIO (CDR)		
SLIDING (CDR > 1.0)			
ECCENTRICITY (CDR > 1.0)			
OVERALL STABILITY (CDR > 1.0)			
BEARING RESISTANCE (CDR > 1.0)			
FACTORED BEARING RESISTANCE (PSF)			

#### **GEOMETRY TABLE**

WALL STATION	ROADWAY STATION	OFFSET TO F.F. WALL	COHESION TOP OF WALL ELEV.	FINISHED GRADE ELEV.

#### SOIL PARAMETERS

- 3				
	SOIL DESCRIPTIONS	TOTAL UNIT WEIGHT (PCF)	FRICTION ANGLE (DEGREES)	COHESION (PCF)
ſ	WALL BACKFILL	120	30	0
ſ	RETAINED SOIL * (INSERT VALUES)		-	
ſ	FOUNDATION SOIL * (INSERT VALUES)		-	

<sup>\*</sup> DESIGN WALL FOR THESE VALUES

#### **GENERAL NOTES**

DRAWINGS SHALL NOT BE SCALED.

THE PLAN QUANTITY FOR THE BID ITEM "WALL MODULAR BLOCK GRAVITY (STRUCTURE)" IS BASED ON A WALL HEIGHT MEASURED FROM THE TOP OF WALL TO A CONSTANT DEPTH OF (INSERT VALUE) BELOW FINISHED GRADE.

AT THE BACK FACE OF THE WALL ALL EXCAVATED VOLUME NOT OCCUPIED BY NEW STOTIOTIES SHALL BE BACKFILLED WITH BACKFILL TYPE A OR GRANULAR BACKFILL BACKFILL BACKFILL SHOWN IN THE TYPICAL SECTION. BACKFILLING SHALL BE INCIDENTAL TO BID ITEM "WALL MODULAR BLOCK GRAVITY (STRUCTURE)". LIMITS OF EXCAVATION SHALL BE DETERMINED BY THE CONTRACTOR.

#### **DESIGN DATA**

THE CONTRACTOR SHALL PROVIDE COMPLETE DESIGN, PLANS, DETAILS, SPECIFICATIONS, AND SHOP DRAWINGS FOR THE RETAINING WALLS IN ACCORDANCE WITH THE SPECIAL PROVISIONS. THE RETAINING WALL MANUFACTURER SHALL PROVIDE TECHNICAL ASSISTANCE TO THE CONTRACTOR DURING CONSTRUCTION. THE COST OF FURNISHING THESE TIEMS SHALL BE INCLUDED IN THE BID ITEM ("INSERT WALL SYSTEM OR SYSTEMS)".

PLANS, ELEVATIONS AND DETAILS SHOWN ON THESE DRAWINGS ARE INTENDED TO INDICATE WALL LOCATIONS, LENGTHS, HEIGHTS, AND DETAILS COMMON TO THE WALL SYSTEM SELECTED. THE CONTRACTOR SHALL VERIFY THAT THE WALL SYSTEM SELECTED WILL CONFORM TO THE REQUIRED ALIGNMENTS AND DETAILS.

THE RETAINING WALL IS TO BE DESIGNED USING THE ELEVATIONS GIVEN ON THIS SHEET

DESIGN FOR RETAINING WALL TO PROVIDE FOR FINISHED GRADE SLOPED BEHIND WALL AS SHOWN.

SEE SPECIAL PROVISIONS FOR AESTHETIC TREATMENT TO WALL.

DESIGN RETAINING WALL FOR A LIVE LOAD SURCHARGE OF (INSERT VALUE).

■ THE CONTRACTOR SHALL COORDINATE POST INSTALLATION WITH THE WALL DESIGN AND WALL DETAILS (WHEN APPLICABLE).

THE MAXIMUM VALUE OF THE ANGLE OF INTERNAL FRICTION OF THE WALL BACKFILL MATERIAL SHALL BE ASSUMED TO BE 30° WITHOUT CERTIFIED TEST VALUES.

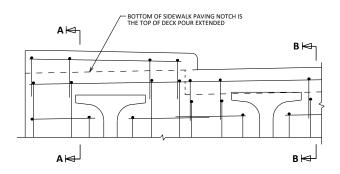
#### **DESIGNER NOTES**

- THE WIDTH PROVIDED IN THE TABLE ARE THE MINIMUM REQUIRED BLOCK WIDTHS BASED UPON THE MINIMUM DESCRIBED IN THE WALL SYSTEM SPECIAL PROVISIONS OR EXTERNAL AND OVERALT STABILITY AT THE DESIGNATE ID CATIONS. THESE DESIGNATED LOCATIONS REPRESENT TYPICAL AND CRITICAL WALL LOCATIONS, BUT SHALL NOT BE CONSIDERED ALL INCLUSIVE. THE CONTRACTOR DESIGN SHALL MEET OR EXCEED THE MINIMUM VALUES REPRESENTED IN THE TABLE AT THESE DESIGNATED LOCATIONS.
- STRATUM LOCATIONS & SOIL DESCRIPTIONS AT EACH BORING LOCATION.
- WHEN APPLICABLE, RAILING OR FENCING SHOULD BE PLACED BEHIND WALL

GRAVITY WALL

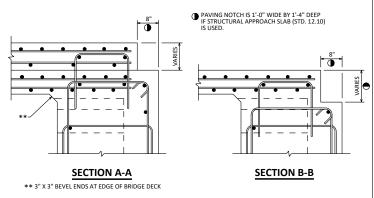


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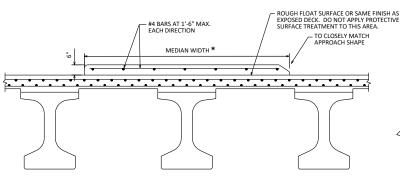


#### PART TRANSVERSE SECTION AT ABUTMENT TYPE A1 DIAPHRAGM WITH A RAISED SIDEWALK

(HORIZ BARS SHOWN ARE THE FERARS DECK REINFORCEMENT NOT SHOWN FOR CLARITY.)



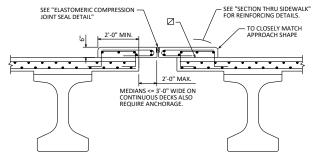
- SEE STANDARDS 19.33, 19.34, 19.35 FOR REINFORCEMENT DETAILS
- DETAILS SHOWN ARE FOR GIRDER STRUCTURES. SIMILAR REINFORCEMENT FOR SLAB STRUCTURES SHALL BE USED WITH A REMINDER THAT THE TRANSVERSE AND LONGITUDINAL REINFORCEMENT LAYERS ARE REVERSED.



#### CROSS SECTION THRU UNANCHORED MEDIAN

\* (ANCHORAGE TO DECK NOT REQUIRED FOR WIDTHS > 3'-0", EXCEPT ALL MEDIAN SECTIONS ON TOP OF PAVING BLOCK MUST BE ANCHORED)

NOTE: CLEAN ALL LOOSE MATERIAL ON THE DECK AT THE MEDIAN LOCATION PRIOR TO MEDIAN PLACEMENT USING HIGH PRESSURE WATER OR AIR, ENSURING ALL FREE-STANDING WATER IS REMOVED PRIOR TO MEDIAN PLACEMENT. NEAT CEMENT IS REQUIRED AS PER 509.3.9.2 OF THE STANDARD SPECIFICATIONS UNLESS THE MEDIAN IS POURED WITHIN 45 DAYS OF COMPLETING THE DECK POLIS



CROSS SECTION THRU MEDIAN WITH A JOINT

# ANCHORED MEDIAN CURB DETAIL

-0

ര 1'-6"

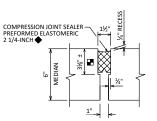
#4 BARS @ 1'-6" MAX.

ANCHORED MEDIAN CURB DETAIL

-0

CONST. JOINT-STRIKE OFF AS SHOWN AND LEAVE ROUGH. FOR DECK POUR, MATCH BRIDGE X-SLOPE.

SEE STD. 24.11 FOR DECK JOINT DETAIL FOR LONGITUDINAL AND TRANSVERSE JOINTS.



#### **ELASTOMERIC COMPRESSION SEAL DETAIL**

- VARIES BASED ON JOINT MANUFACTURER
- ◆ MANUFACTURER SHALL LABEL TOP OF SEAL

SUPERELEVATIONS THE DECK MAY BE LEVEL BENEATH
THE SIDEWALK (MAINTAIN CONSTANT DECK THICKNESS)

WHEN PARAPETS ARE POURED CONTINUOUSLY FROM END TO END, THEY SHALL BE SEPARATED AT THE DEFLECTION JOINTS BY A PIECE OF 1/2" ZINC OR PLASTIC PLATE CUT AS SHOWN IN THE "DEFLECTION JOINT DETAIL". IF CONSTRUCTION JOINTS IN PARAPETS ARE USED AT THE DEFLECTION JOINTS, ONE SIDE OF JOINT SHALL BE COATED WITH AN APPROVED LIQUID BOND BREAKER AND PLATE SEPARATORS MAY BE OMITTED

- CONST. JOINT-STRIKE OFF AS SHOWN AND LEAVE ROUGH. FOR DECK POUR, MATCH BRIDGE X-SLOPE.
- 8" MIN. SIDEWALK THICKNESS ALSO REQ'D AT EDGE OF DECK/SLAB.
- ± 0.5% CONSTRUCTION TOLERANCE IN SIDEWALK CROSS SLOPE. THE SIDEWALK CROSS SLOPE SHALL NOT EXCEED 2% WITHOUT PRIOR APPROVAL FROM THE ENGINEER.

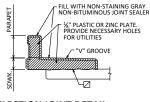
#### **DESIGNER NOTES**

FOR EXTREME SIDEWALK WIDTHS AND/OR TO REDUCE EXCESSIVE SIDEWALK THICKNESS.

FOR DEAD LOAD PURPOSES, THE SUPERSTRUCTURE DESIGN SHALL ACCOUNT FOR A MAXIMUM 2% SIDEWALK CROSS SLOPE.

#### 1'-3" LEVEL PARAPET/RAIL REQUIREMENTS SAME AS FOR A BRIDGE WITHOUT A RAISED SIDEWALK #5 BARS AT 4" CTRS WITH STANDARD HOOK #4 BARS AT 9"± CTRS. USE CLASS 'C' LAP - 2½" CL. (EXTEND 1'-0" PAST $\square$ SLOPE 1.5% A EDGE OF DECK) Ľ#4 RARS 0 SEE STD 17.02 → FOR ¾" V-GROOVE DETAILS #4 BARS AT 6" CTRS. (WITH 1'-0" LEGS) 2'-0" MAX -C/L GIRDER 5'-0" MAX.

SECTION THRU SIDEWALK



#### **DEFLECTION JOINT DETAIL**

SHOW DEFLECTION JOINT IN PARAPET OR SIDEWALK USING THE FOLLOWING CRITERIA:

1. GIRDER STRUCTURES AND SLAB STRUCTURES WITH A RAISED SIDEWALK SHOULD HAVE A DEFLECTION JOINT IN THE SIDEWALK AND PARAPET OVER THE PIER. FOR SKEWS GREATER THAN 20°, DETAIL THE JOINT NORMAL TO THE SIDEWALK AND PARAPET WITH THE JOINT APPROX CENTERED OVER C/L PIER.

IF THERE IS A LIGHT STANDARD AT THE PIER, PLACE A DEFLECTION JOINT APPROX. 4'-0" EACH SIDE OF PIER, WITH NONE DIRECTLY OVER

2. GIRDER STRUCTURES AND SLAB STRUCTURES WITHOUT SIDEWALKS SHOULD HAVE NO DEFLECTION JOINTS IN THE PARAPETS.

**MEDIAN AND RAISED** SIDEWALK DETAILS

- EDGE OF MEDIAN

ADHESIVE ANCHORS NO. 4 BAR.

- EDGE OF MEDIAN - 2" R.

ADHESIVE ANCHORS NO. 4 BAR

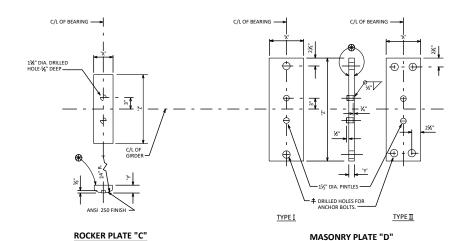


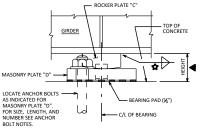
APPROVED: Laura Shadewald

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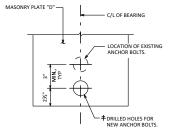
STANDARD 17.01

LENGTH OF	TOTAL	PLATE C		PLATE D			HEIGHT	
PLATE "C"	LOAD KIPS	Х	Υ	Z	х	Υ	Z	FEET
10"	215	5"	23/8"	10"	8"	1¾"	1'-7"	0.354
12"	260	5"	23/8"	1'-0"	9"	1¾"	1'-9"	0.354
12	280	5"	23/8"	1'-0"	10"	23/8"	1'-9"	0.406
	280	5"	115/16"	1'-2"	9"	1¾"	1'-11"	0.318
14"	335	5"	2¾"	1'-2"	11"	23/8"	1'-11"	0.406
14	385	5"	23/8"	1'-2"	1'-1"	21/8"	1'-11"	0.448
	410	5"	2¾"	1'-2"	1'-3"	27/8"	2'-0"	0.448
	275	5"	115/16"	1'-4"	8"	1¾"	2'-1"	0.318
	330	5"	115/16"	1'-4"	10"	23/8"	2'-1"	0.370
16"	390	5"	2¾"	1'-4"	1'-0"	23/8"	2'-1"	0.406
	465	5"	2¾"	1'-4"	1'-2"	21/8"	2'-2"	0.448
	490	5"	2¾"	1'-4"	1'-4"	3¾"	2'-2"	0.490
	325	5"	11½6"	1'-6"	9"	1¾"	2'-3"	0.318
	390	5"	115½"	1'-6"	11"	23/8"	2'-3"	0.370
18"	465	5"	2¾"	1'-6"	1'-1"	27/8"	2'-4"	0.448
	495	5"	2¾"	1'-6"	1'-2"	27/8"	2'-4"	0.448
	560	5"	2¾"	1'-6"	1'-4"	3¾"	2'-4"	0.490
	350	5"	11½"	1'-8"	9"	1¾"	2'-5"	0.318
	380	5"	11½"	1'-8"	10"	23/8"	2'-5"	0.370
20"	460	5"	2¾"	1'-8"	1'-0"	23/8"	2'-6"	0.406
20	530	5"	2¾"	1'-8"	1'-2"	27/8"	2'-6"	0.448
	600	5"	2¾"	1'-8"	1'-4"	3¾"	2'-6"	0.490
	640	5"	23/8"	1'-8"	1'-6"	31/8"	2'-6"	0.531
	405	5"	1 <sup>15</sup> / <sub>16</sub> "	1'-10"	10"	23/8"	2'-7"	0.370
	490	5"	1 <sup>15</sup> / <sub>16</sub> "	1'-10"	1'-0"	23/8"	2'-8"	0.370
22"	565	5"	2¾"	1'-10"	1'-2"	27/8"	2'-8"	0.448
22	635	5"	2¾"	1'-10"	1'-4"	3¾"	2'-8"	0.490
	705	5"	2¾"	1'-10"	1'-6"	37/8"	2'-8"	0.531
	720	5"	2¾"	1'-10"	1'-8"	37/8"	2'-8"	0.531





# FIXED BEARING ASSEMBLY (SEE "DESIGNER NOTES" FOR BEARING REPLACEMENTS)



MASONRY PLATE "D"
BEARING REPLACEMENTS

AT FIXED BRG.

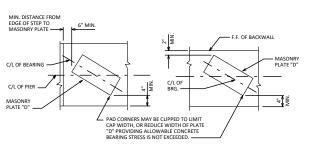
#### **ANCHOR BOLT NOTES**

FOR SPAN LENGTHS UP TO 100'-0": USE A TYPE I MASONRY PLATE "D" WITH (2) - 1½" DIA. x 1'-5" LONG ANCHORS BOLTS.

FOR SPAN LENGTHS FROM 100'-0" UP TO 150'-0": USE A TYPE I MASONRY PLATE "D" WITH (2) -  $1\frac{1}{2}$ " DIA. x 1'-10" LONG ANCHORS BOLTS.

FOR SPAN LENGTHS GREATER THAN 150'-0": USE A TYPE  $\overline{1}$  MASONRY PLATE "D" WITH (4) -  $1\frac{1}{2}$ " DIA. x 1'-10" LONG ANCHORS BOLTS.

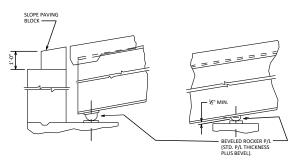
CHECK THAT ANCHOR BOLTS PROVIDE ADEQUATE HORIZONTAL CAPACITY.



AT SKEWED PIER

AT SKEWED ABUTMENTS

CLEARANCE DIAGRAM



AT EXPANSION BRG.

**BEVELED ROCKERS WITH GRADES GREATER THAN 3%** 

#### BEARING NOTES

ALL BEARINGS ARE SYMMETRICAL ABOUT C/L OF GIRDER AND C/L OF BEARING.

IN LIEU OF USING SHIM PLATES, FABRICATOR MAY INCREASE THICKNESS OF MASONRY PLATE "D" BY THE SHIM PLATE THICKNESS.

ALL STRUCTURAL STEEL BEARING PLATES SHALL BE FLAT ROLLED STEEL PLATES WITH ALL SURFACES SMOOTH AND FREE FROM WARP AND ALL EDGES SMOOTH, STRAIGHT AND VERTICAL.

ALL PLATE CUTS SHALL BE MACHINE OR MACHINE FLAME CUTS

ALL FINISHED SURFACES SHALL BE MACHINE FINISHED BY AN AUTOMATIC PROCESS.

ANCHOR BOLTS SHALL BE THREADED 3". PROVIDE ONE STANDARD WROUGHT WASHER AND ONE HEX NUT PER BOLT. PROJECT ANCHOR BOLTS, MASONRY PLATE "D" THICKNESS + 2½", ABOVE TOP OF CONCRETE.

ALL MATERIAL IN BEARINGS, INCLUDING SHIM PLATES, BUT EXCLUDING PINTLES, ANCHOR BOLTS, NUTS AND WASHERS SHALL CONFORM TO ASTM A709 GRADE 50W.

STEEL PINTLES SHALL CONFORM TO ASTM A449 OR ASTM A572 GRADE 50.

ALL MATERIAL IN TYPE "A" BEARINGS, INCLUDING SHIM PLATES AND BEARING PADS, SHALL BE PAID FOR AT THE UNIT PRICE BID FOR "BEARING ASSEMBLIES FIXED B-\_-", EACH.

CHAMFER TOP OF PINTLES  $\frac{1}{8}$ ". DRILL HOLES FOR ALL PINTLES IN MASONRY PLATE "D" FOR A DRIVING FIT.

PROVIDE  $\frac{1}{N}$  THICK BEARING PAD THE SAME SIZE AS MASONRY PLATE "D" FOR EACH BEARING.

CHAMFER ANCHOR BOLTS PRIOR TO THREADING.

ANCHOR BOLTS, NUTS AND WASHERS SHALL CONFORM TO ASTM F1554 GRADE 55, OR MATERIAL OF EQUIVALENT YIELD STRENGTH AND ELONGATION.

ANCHOR BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM A153, CLASS C.

ROCKER PLATE "C" SHALL BE SHOP PAINTED WITH A WELDABLE PRIMER.

MASONRY PLATE "D" SHALL BE GALVANIZED.

PLACE SHIM PLATES BETWEEN BEARING PAD AND MASONRY PLATE "D". PLATES SHALL HAVE 'X' AND 'Z' DIMENSIONS THAT MATCH MASONRY PLATE "D".

PRILLED HOLES FOR ANCHOR BOLTS IN MASONRY PLATE "D" SHALL HAVE A
 DIAMETER ¾" LARGER THAN ANCHOR BOLT.

FINISH THESE SURFACES TO ANSI 250 IF 'Y' DIMENSION IS GREATER THAN 2".

#### **DESIGNER NOTES**

HEIGHT OF BEARINGS GIVEN IN TABLE INCLUDES ½" BEARING PAD

DETAIL SHIM PLATES AS DESCRIBED IN NOTES ON STANDARD 24.02.

REFER TO THE DETAILS BELOW FOR THE USE OF BEVELED ROCKER PLATE "C" ON GRADES GREATER THAN 3% AND ALSO CLEARANCE REQUIREMENTS.

FOR WELD SIZE, REFER TO STANDARD 24.02

ADJUST HEIGHT IF BEVELED ROCKER PLATE "C" IS USED.

FOR BEARING REPLACEMENTS, DESIGNER SHALL UTILIZE A WIDER BEARING THAN THE EXISTING GIRDER BOTTOM FLANCE WIDTH TO ALLOW FOR FIELD WELDING OF THE EDGE OF THE BOTTOM FLANGE TO THE TOP OF PLATE "C". SEE STANDARD 40.08 FOR DETAILS.

CALCULATE THE REACTION AT THE BEARINGS DUE TO "TOTAL LOADS". USE THE AASHTO LRFD SERVICE I LOAD COMBINATION. CONSIDER ONLY DEAD LOAD (DC + DW) AND HL-93 LIVE LOADS (LL), INCLUDING A 33% DYNAMIC LOAD ALLOWANCE (IM).

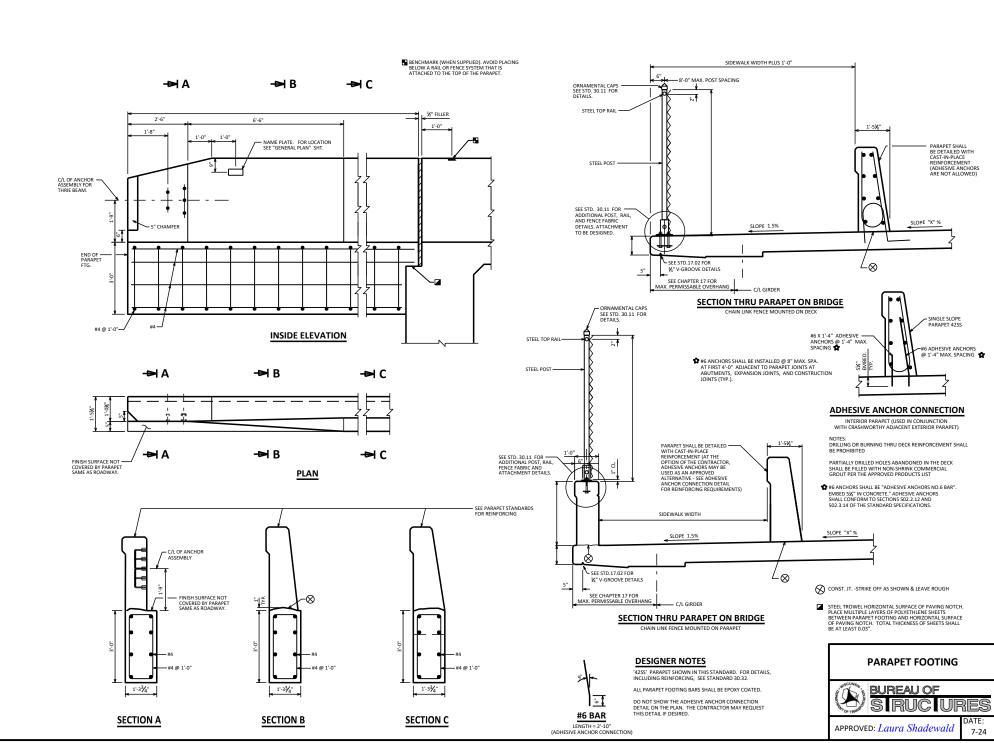
THE VALUES IN THE TABLES ARE THE BEARING CAPACITIES FOR "TOTAL LOAD" (DC + DW + (LL + IM)).

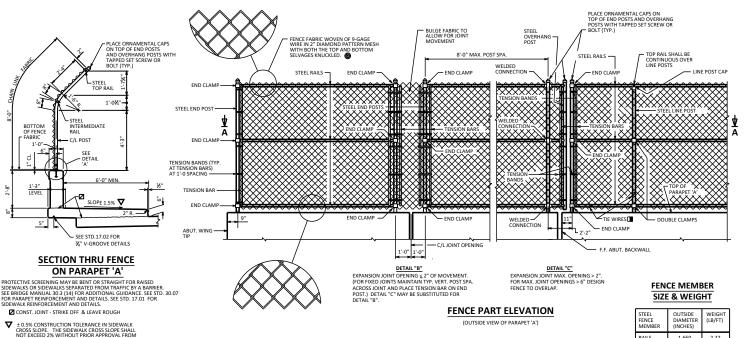
SELECT A BEARING THAT HAS A CAPACITY GREATER THAN OR EQUAL TO THE CALCULATED REACTION FOR "TOTAL LOADS".

FIXED BEARING DETAILS
TYPE 'A' - STEEL GIRDERS



APPROVED: Laura Shadewald





STEEL FENCE MEMBER	OUTSIDE DIAMETER (INCHES)	WEIGHT (LB/FT)
RAILS	1.660	2.27
END POST	2.875	5.80
OVERHANG POST	2.875	5.80
LINE POST	2.375	3.65
POST SLEEVE	4.000	9.12

¾" DIA. GALV. CARRIAGE BOLT WITH LOCKING

RΔII

Ø<sub>16</sub>//

ANCHOR PLATE

- A ANCHOR BOLT

NUT. (TO BE SUPPLIED WITH ASSEMBLY)

FILL SLEEVE AND BEVEL AWAY FROM

C/L POST

SLOPE GROUT FOR DRAINAGE

PARAPET

'A' OR 'SS'

TACK WELD

@ 1/3 POINTS

POST

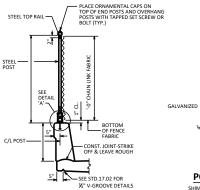
SLEEVE

POST WITH NON-SHRINK GROUT AFTER -SETTING POST. (LEAVE NO VOIDS)

WELD 1½" X ½" X 2" LONG LUG TO POSTS BRACE BAND STEEL END POST OVERHANG POST OR POST SI FEVE RAIL END RAIL END 1/4 - STEEL RAIL STEEL RAIL C/L POST GRIND RAIL TO BRACE BAND ¾" DIA. X 1¼" GALV. CARRIAGE BOLT. (TYP.) OR POST SLEEVE STEEL RAIL RAII FND END CLAMP \* DOUBLE CLAMP WELDED CONNECTION DRILL  $rac{N}{6}$ " DIA. DRAIN HOLE PARALLEL TO ROADWAY IMMEDIATELY ABOVE GROUT IN POST. SLEEVE LOCATIONS ONLY. STEEL END POST (AT OVERHANG SECTION) **SECTION A-A** 

NOTE: PLACE ALL BOLT HEADS ON SIDE OF

FENCE ADJACENT TO PEDESTRIANS



**SECTION THRU FENCE** 

ON SINGLE SLOPE PARAPET

FOR TRAFFIC BARRIER APPLICATION, USE STRAIGHT POSTS (NOT BENT POSTS)

WEIGHT OF CHAIN LINK FENCE: (BASED ON 8 FT. POST SPACING)

6 FT. HIGH FENCE = 18 LB / FT

8 FT. HIGH FENCE = 21 IB / FT

21/4" 21/4 POST SHIM DETAILS SHIMS REQUIRED ONLY WHEN END POSTS AND LINE POSTS ARE WELDED TO BASE PLATES. PROVIDE 4 SHIMS PER POST. USE WHERE REQUIRED FOR ALIGNMENT.

FIELD CLIP AS REQ'D. -

GALVANIZED Θ C/L POST ½" x 2" x 8"-ANCHOR PLATE

NOTE: ANCHOR PLATE NOT REQUIRED

%" DIA. HOLE FOR 1/2" DIA ANCHOR BOLTS POST SLEEVE  $\ominus$ LINE POST  $\oplus$ - C/L FENCE POST Θ C/L POST -

**BASE PLATE** 

DETAIL 'A' UNIT SHALL BE GALVANIZED AFTER FABRICATION

NOTE: IN LIEU OF USING THE POST SLEEVE, THE FENCE POST MAY BE WELDED TO THE BASE PLATE.

#### NOTES

POSTS ARE TO BE SET VERTICAL

METALLIC-COATED FENCE SYSTEM: ALL FENCE COMPONENTS SHALL BE GALVANIZED STEEL, EXCEPT THE FENCE FABRIC WHICH MAY BE ALUMINUM- COATED STEEL OR

FABRIC SHALL CONFORM TO ASTM A491 OR A392, CLASS 2. STEEL RAILS. POSTS AND POST SLEEVES SHALL CONFORM TO ASTM F1083 STANDARD WEIGHT PIPE (SCHEDULE 40). FITTINGS SHALL CONFORM

THE BID ITEM SHALL BE "FENCE CHAIN LINK - FT."

POLYMER-COATED FENCE SYSTEM:
ALL FENCE COMPONENTS SHALL BE GALVANIZED STEEL WITH A COLORED POLYMER-COATING ON THE OUTSIDE.

FABRIC SHALL CONFORM TO ASTM F668, CLASS 2B. STEEL RAILS, POSTS AND POST SLEEVES SHALL CONFORM TO ASTM F1083, STANDARD WEIGHT PIPE (SCHEDULE 40). FITTINGS SHALL CONFORM TO ASTM F626. SEE THE "BRIDGE SPECIAL PROVISIONS" FOR ADDITIONAL DETAILS.

THE COLOR OF POLYMER-COATING FOR THIS STRUCTURE SHALL BE [SPECIFY: DARK GREEN, BROWN OR BLACK] IN ACCORDANCE WITH ASTM F934.

THE BID ITEM SHALL BE "FENCE CHAIN LINK POLYMER - COATED

COMPLETE ANY REQUIRED WELDING OF COMPONENTS BEFORE

POST BASE PLATES SHALL BE FLAT WITH ALL SURFACES SMOOTH AND FREE FROM WARP AND ALL EDGES SMOOTH. STRAIGHT AND VERTICAL. ALL PLATE CUTS SHALL BE MACHINE OR MACHINE

BASE PLATES, ANCHOR PLATES AND SHIMS SHALL BE ASTM A709

ALL POST SPACINGS ARE MEASURED HORIZONTALLY ALONG THE C/L OF THE POST.

- CAULK AROUND PERIMETER OF BASE PLATE AND FILL PORTION OF SLOTTED HOLE AROUND ANCHOR BOLT IN SHIM WITH NON-STAINING GRAY NON-BITUMINOUS JOINT SEALER.
- \* ALTERNATE TO DOUBLE CLAMP: USE LINE RAIL CLAMP (BOULEVARD)
  OR 180° BRACE BAND, WHICH MAY BE USED WHEN THE POSTS ARE
  EITHER BOLTED TO THE POST SLEEVES OR DIRECTLY WELDED TO THE BASE PLATE.
- ▲ ANCHOR BOLTS, NUTS AND WASHERS SHALL BE EITHER STAINLESS STEEL OR ASTM 307. IF 307 IS USED, ANCHOR BOLTS, NUTS, AND WASHERS SHALL BE GALVANIZED.
- ☆ ALTERNATIVE ANCHORAGE: CONCRETE ADHESIVE ANCHORS ½"-INCH. EMBED 7" IN CONCRETE. ADHESIVE ANCHORS SHALL CONFORM TO SECTIONS 502.2.12 AND 502.3.14 OF THE STANDARD SPECIFICATIONS.
- ATTACH FABRIC TO RAILS, AND TO POSTS WITHOUT TENSION BANDS, VITH TIE WIRES (ROUND, 9-GAGE) SPACED AT 1'-0"
- BOLT RAIL TO RAIL END TO SECURE OVERHANG SECTION ALTERNATE IS TO WELD RAIL DIRECTLY TO END POST.

MINIMUM LENGTH OF TOP RAIL BETWEEN SPLICES SHALL BE 20'-0". LOCATE SPLICES NEAR ½ POINT OF POST SPACING.

#### **DESIGNER NOTES**

THE CHAIN LINK FENCE SYSTEM SELECTED FOR THE STRUCTURE SHALL BE A "METALLIC-COATED FENCE SYSTEM" OR A "POLYMER-

VULNERABLE AREAS, OR AS STATED IN FDM PROCEDURE 11-35-1 FOR PROTECTIVE SCREENING.

PEDESTRIAN RAILING MAY BE USED ON WINGWALL PARAPETS IF CHAIN LINK FENCE DOES NOT CONTINUE BEYOND BRIDGE.

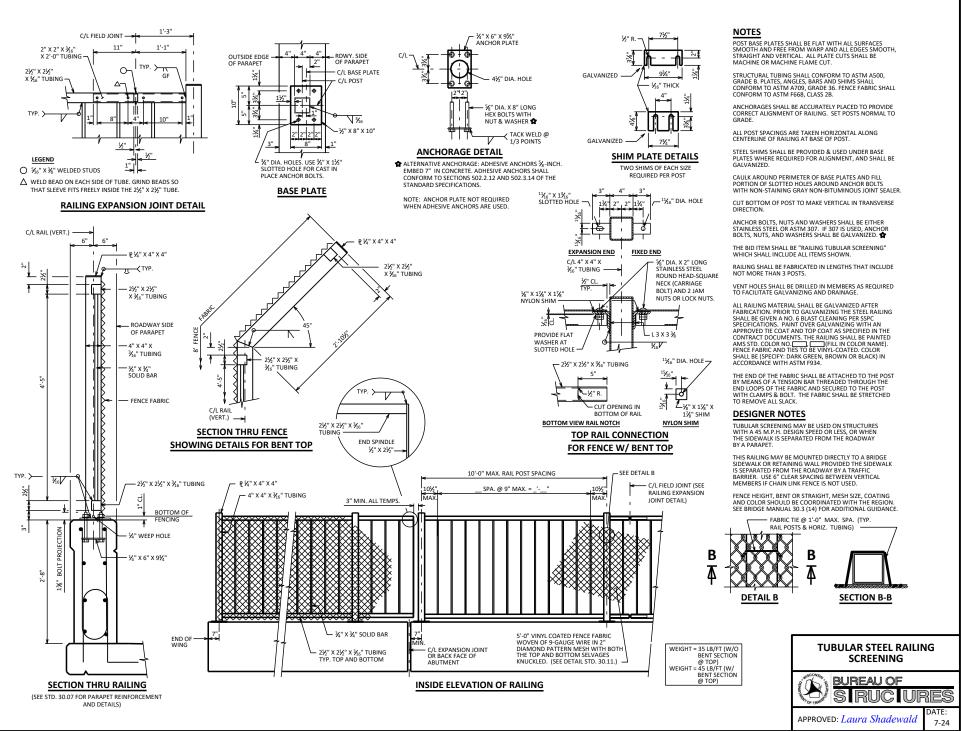
HANDRAILS SHALL BE USED ALONG BRIDGE SIDEWALKS WHERE THE SLOPE OF THE SIDEWALK IS GREATER THAN 5%. TOP OF HANDRAIL GRIPPING SURFACES SHALL BE MOUNTED BETWEEN 30" & 34" ABOVE SIDEWALK SURFACE. USE 30" NEAR SCHOOL ZONES, IF FEASIBLE. HANDRAILS SHALL BE PROVIDED ALONG BOTH SIDES OF SIDEWALK FOR HANDRAIL DETAILS SEE STANDARD 37.02.

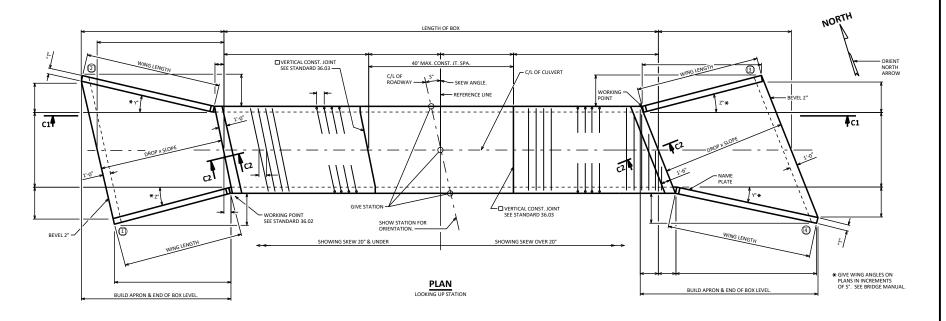
FOR DEAD LOAD PURPOSES. THE SUPERSTRUCTURE DESIGN SHALL ACCOUNT FOR A MAXIMUM 2% SIDEWALK CROSS SLOPE.

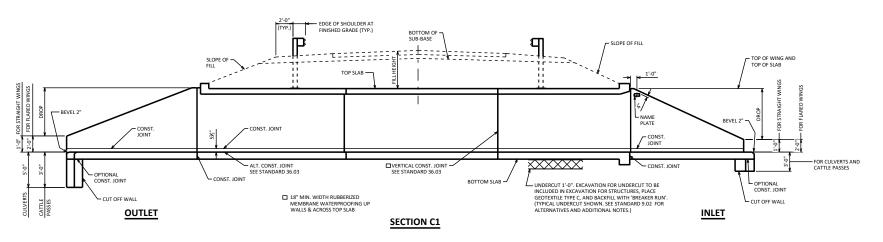




APPROVED: Laura Shadewald







#### INDICATES WING NUMBER LIVE LOAD: DESIGN LOADING: HI -93 INVENTORY RATING FACTOR: RF=1.\_\_ OPERATING RATING FACTOR: RF=1.\_\_ WISCONSIN STANDARD PERMIT VEHICLE (WIS.-SPV): \_ \_ \_ (KIPS) \*\* DESIGNED FOR FILL HEIGHT RANGE OF\_TO\_FEET

LEGEND

**DESIGN DATA** 

MATERIAL PROPERTIES: CONCRETE MASONRY. BAR STEEL REINFORCEMENT fy = 60,000 P.S.I.

#### NOTES

SEE STANDARD 36.02 FOR NOTES.

#### **DESIGNER NOTES**

FOR SECTION C2 AND CONST. JOINT DETAILS SEE STANDARD 36.03

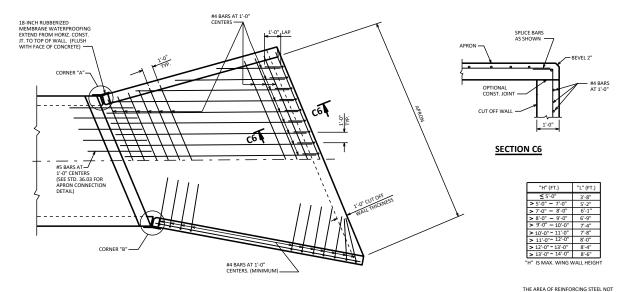
- \* \* SEE SECTION 36.5 FOR DESIGN RANGE OF FILL HEIGHTS. HEIGHT TO BE TO THE NEAREST 0.5 FEET ON FILLS UNDER 4 FEET AND TO THE NEAREST FOOT ON
- SEE STANDARD 36.02 FOR ADDITIONAL DESIGNER NOTES.
- SEE CHAPTER 45 FOR LOAD RATING OF EXISTING CONCRETE BOX CULVERTS.

LOCATE THE NAME PLATE ON THE FIRST RIGHT WING TRAVELING IN THE HIGHWAY CARDINAL DIRECTIONS OF NORTH OR EAST.





APPROVED: Laura Shadewald



APRON DETAIL

#### NOTES

BAR STEEL REINFORCEMENT SHALL BE EMBEDDED 2" CLEAR UNLESS OTHERWISE SHOWN OR

THE CONCRETE IN THE CUT OFF WALL MAY BE PLACED UNDERWATER IF THE EXCAVATION CANNOT BE DEWATERED.

THE "ALTERNATE CUT OFF WALL" DETAIL SHOWN ON THIS SHEET MAY BE USED IN LIEU OF THE CAST-IN-PLACE CONCRETE CUT OFF WALLS. PAYMENT SHALL BE BASED ON CONCRETE CUT OFF WALLS.

#### **DESIGNER NOTES**

IF PRECAST ELEMENTS ARE ALLOWED, INCLUDE THE FOLLOWING NOTE ON THE LAYOUT SHEET:

THE CONTRACTOR MAY FURNISH (INSERT ALLOWABLE PRECAST ELEMENTS). IN LEU OF THE CAST IN-PACE ROOK CLUVERT WITH HE ACCEPTANCE OF THE SHOP BOWNINGS BY THE STRUCTURES MAINTENANCE SECTION. THE PRECAST CONCRETE BOX CLUVERT SHALL CONCORNO THO TRECAST DETAILS IN CHAPTER 35 STANDARDS OF THE LUTRENT WISCONSIN DOT BRIDGE MANUAL PAYMENT FOR THE PRECAST CULVERT SYSTEM SHALL BE BASED ON THE QUANTITIES AND PRICES BID FOR THE ITEMS LESS IN THE "CHAPTERS STANDERS" IN THE "CHAPTERS" OF THE CHAPTER STANDERS OF THE CHAPTER STANDERS OF THE THE MAIL SHE RECEIVED FOR THE THE STANDERS OF THE

ALLOWABLE PRECAST ELEMENTS INCLUDE: BOX CULVERT BARREL SECTIONS, WINGWALLS, HEADERS, AND CUTOFF WALLS. APRON FLOORS SHALL BE CAST-IM-PLACE, UNILESS DESIGNED OTHERWISE. THE DESIGNERS HALL DESTERNINE IF PRECAST ELEMENTS ARE ALLOWED ON A PROJECT-IM-PROJECT BASS, PRECAST ONLY DESIGNS REQUIRE PRIOR APPROVAL BY THE BUREAU OF STRUCTURES. WHEN PRECAST ELEMENTS HAVE BEEN DETERMINED TO BE PROHIBITED, ELEMENTS SHALL BE NOTED ACCORDINGLY ON THE PLANS (E.G. "A PRECAST WINGWALL ALTERNATIVES IN OTHER LOWED").

PROVIDE CAST-IN-PLACE DETAILS ONLY, UNLESS SPECIAL PRECAST DETAILS ARE REQUIRED OR WHEN A PRECAST ONLY DESIGN IS PROVIDED.

PRECAST ONLY DESIGNS REQUIRE PRIOR APPROVAL BY THE BUREAU OF STRUCTURES. SEE BRIDGE MANUAL SECTIONS 36.11.4 AND 36.12 FOR ADDITIONAL INFORMATION. IF USED, PROVIDE PRECAST DETAILS FOLUCINING STANDARDS 36.05 AND 36.66 WITH THE FOLLOWING SPECIFICATIONS:

PRECAST CONCRETE WINGWALLS (STRUCTURE) (504.1000.5)

PRECAST CONCRETE WINGWALLS (STRUCTURE) (504.1000.S)
PRECAST CONCRETE BOX CULVERT, (SPAN SIZE) FT X (RISE SIZE) FT (504.2000.S)

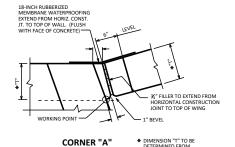
ALL BAR STEEL FOR CAST-IN-PLACE CONCRETE BOX CULVERTS SHALL BE UNCOATED, EXCEPT WHEN FILL IS LESS THAN 2-FT WHILE SUPPORTING TRAFFIC LOAD, EPOXY COATED BARS SHALL BE USED FOR THE TOP AND BOTTOM BARS IN THE TOP SLAB, PRECAST BOX CULVERT NOT ALLOWED FOR WHEN FILL IS LESS THAN 2-FT WHILE SUPPORTING TRAFFIC LOAD.

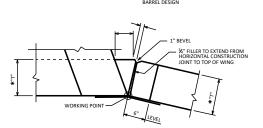
BAR STEEL FOR CAST-IN-PLACE CONCRETE APRONS SHALL BE UNCOATED AND BAR STEEL FOR WINGWALL DOWLES AND ALL WINGWALL BARS SHALL BE EPOXY COATED (FOR CAST-IN-PLACE AND PRECAST WINGSWALLS).

FOR "B" DESIGNATED CONCRETE BOX CULVERTS HAVING THEIR TOP SURFACE AT GRADE, HAND HELD FINISHING MACHINES MAY BE USED. NOTE THIS ON PLANS WHEN APPLICABLE.

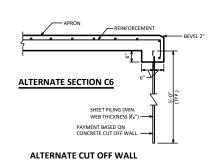
SEE STANDARDS 9.02 AND 36.01 FOR ADDITIONAL NOTES.

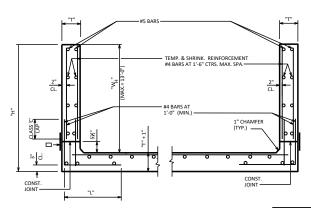
SEE STANDARDS 36.05 AND 36.06 FOR PRECAST BOX CULVERT DETAILS.





CORNER "B"





IDENTIFIED IN SECTIONS SHALL CONFORM
TO THE FOLLOWING TEMPERATURE AND
SHRINKAGE REQUIREMENTS:

THICKNESS T&S REINF.

≤ 12" #4 @ 18"

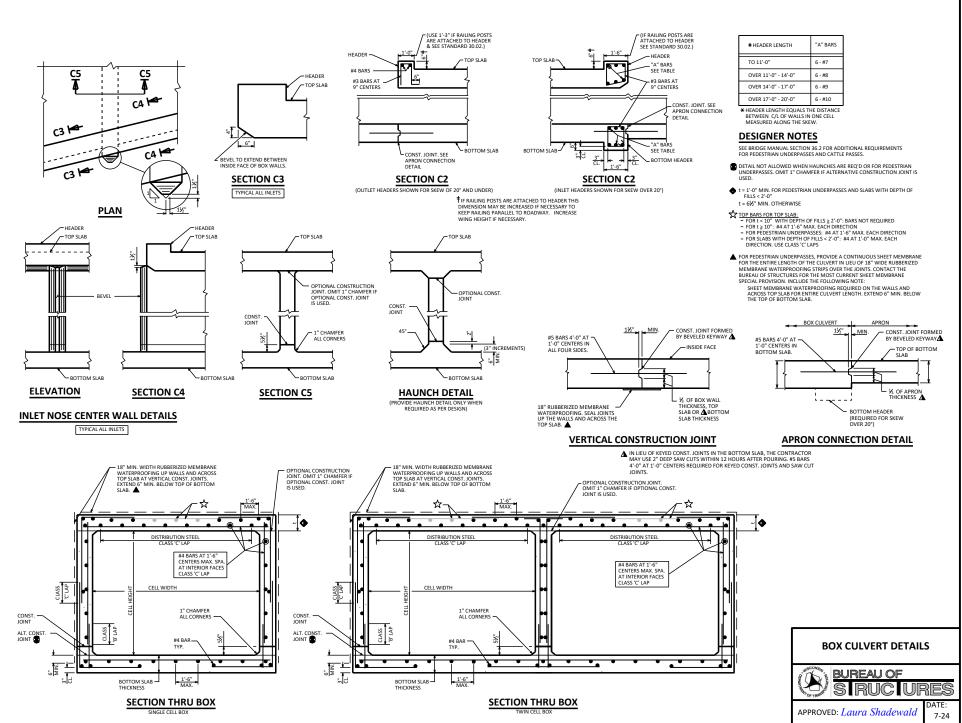
> 12" - 18" #4 @ 12"

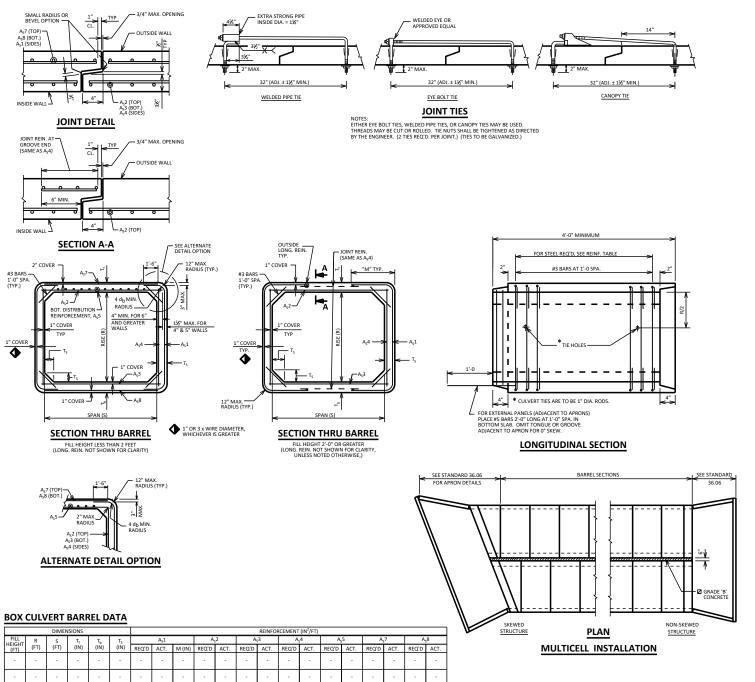
#### SECTION THRU WINGWALLS

■ 18" MIN. WIDTH RUBBERIZED MEMBRANE WATERPROOFING ALONG HORIZ. CONSTR. JT. IN WING.

#### **BOX CULVERT APRON DETAILS**







#### NOTES

DETAILS FOR MATERIALS, FABRICATION, CONSTRUCTION AND DESIGN OF PRECAST BOX CULVERTS NOT SHOWN OR STATED ON THIS DRAWING SHALL BE IN ACCORDANCE WITH THE CURRENT ASTM SPECIFICATION CL577, ASHTOL HEP BRIDGE DESIGN SPECIFICATIONS, WISCONSIN DOT BRIDGE MANUAL; WISCONSIN DOT STANDARD SPECIFICATIONS & APPLICABLE SPECIAL PROVISIONS, EXCEPT THAT THE CONCRETE MIXTURE SHALL CONTAIN NOT LESS THAN 556 JBS. OF CEMENTIFIOUS MATERIALS PER CUBIC YARD.

THE DESIGN OF PRECAST BOX CULVERTS WITH ALL FILL HEIGHTS SHALL BE AS STATED IN ASTM C1577.

THE JOINT ON ALL SIDES OF THE CUIVERT SHALL BE SEALED WITH A PREFORMED BLUTY RUBBER SEALANT IN CONFORMANCE WITH ASTM (1990 SECTION 6.2 . A 2'-0" STIRP OF GEOTEXTILE TYPE OF SCHEDULE A SHALL BE PLACED OVER THE JOINT SON THE TOP AND ON THE SIDES OF THE CUIVERT. THE GEOTEXTILE SHALL CONFORM TO SECTION 6.9.2.2.4 OF THE STANDARD SPECIFICATION, (FABRIC NOT REQUIRED OVER INSIDE WALL JOINTS OF MULTICELL INSTALLATION.)

PRECAST CONCRETE SECTIONS SHALL BE PLACED ON A BEDDING OF "STRUCTURE BACKFILL TYPE B" OF 6" MINIMUM DEPTH AND AS APPROVED BY THE ENGINEER.

THE COVER OF CONCRETE OVER THE REINFORCEMENT SHALL BE 1 INCH OR 2 INCHES AS SHOWN WITH AN ALLOWABLE VARIATION OF  $\frac{1}{2}$ 8" TO  $\frac{1}{2}$ 2".

THE SPACING CTR. TO CTR. OF THE CIRCUMFERENTIAL WIRES SHALL NOT BE LESS THAN 2 INCHES NOR MORE THAN 4 INCHES. THE SPACING CTR. TO CTR. OF THE LONGIT. WIRES SHALL NOT BE MORE THAN 8 INCHES. PROVIDE 0.03 SQ. IN./FT MINIMUM LONG. REINFORCEMENT AT EACH FACE IN SLABS AND WALLS.

NOT MORE THAN FOUR (4) HOLES MAY BE CAST, DRILLED OR OTHERWISE NEATLY MADE IN THE SHELL OF EACH PIECE OF BOX SECTION FOR HANDLING. THE HOLES SHALL BE TAPERED UNLESS DRILLED. HOLES SHALL BE FILLED WITH PORTLAND CEMENT MORTAR EXCEPT TAPERED HOLES MAY BE FILLED WITH FOR CONCRETE PULGS SECURED WITH PORTLAND CEMENT MORTAR OR OTHER APPROVED ADHESIVE.

WHEN TWO OR MORE BARRELS ARE UTILIZED IN PARALLEL FOR MULTICELL INSTALLATIONS THE CLEAR SPACING BETWEEN BARRELS SHALL BE 6 INCHES AND THE SPACE BETWEEN ADJACKIN BARRELS FROM TOP OF BEDDING TO TOP OF TOP SLAB SHALL BE FILLED WITH GRADE "B" CONCRETE.

SHOP DRAWINGS SHALL PROVIDE "BOX CULVERT BARREL DATA" WITH REQUIRED AND ACTUAL REINFORCEMENT AREAS.

#### MATERIAL PROPERTIES:

F	PRECAST CONCRETE -	f'c = 5,000 P.S.I.
E	BAR STEEL REINFORCEMENT ———	— fy = 60,000 P.S.I.
	STEEL REINFORCEMENT (WIRE)	- fv = 65 000 P S I

#### DESIGNER NOTES:

PROVIDE PRECAST DETAILS WHEN SPECIAL DETAILS ARE REQUIRED OR WHEN A PRECAST ONLY DESIGN IS PROVIDED. SEE STD. 36.02 FOR ADDITIONAL INFORMATION.

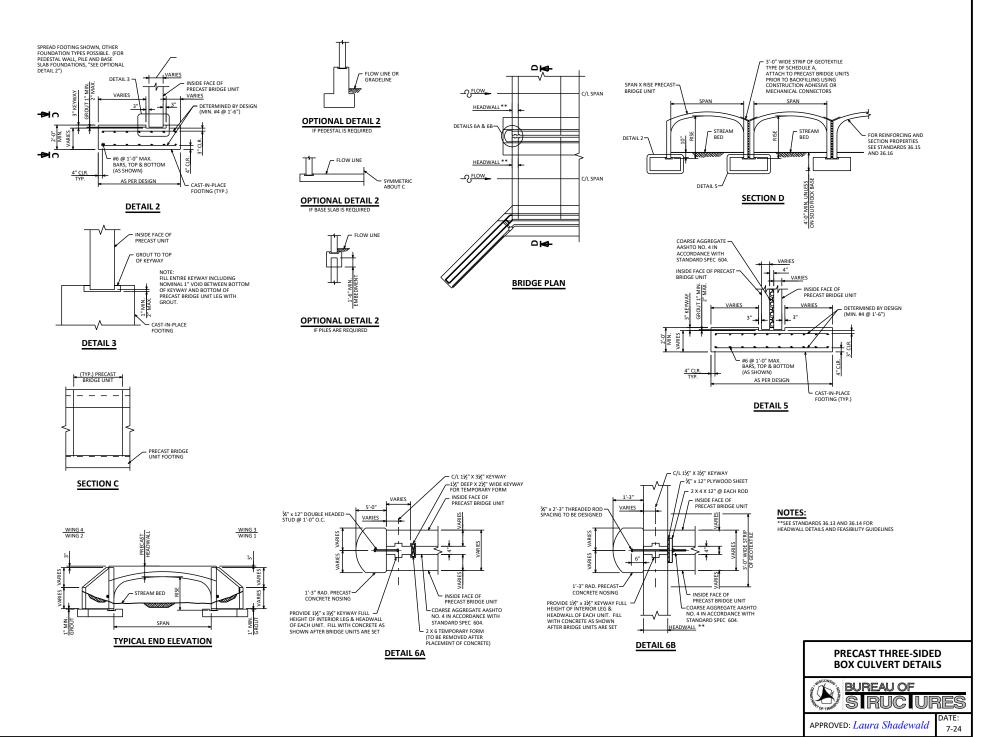
PROVIDE "BOX CULVERT BARREL DATA" ON CONTRACT PLANS WHEN BOX CULVERT BARREL SECTIONS WARRANT DESIGN REQUIREMENTS BEYOND ASTM C1577 "TABLE A". SEE BRIDGE MANUAL SECTION 36.12 FOR SPECIAL CONDITIONS WARRANTING A SEPARATE ANALYSIS.

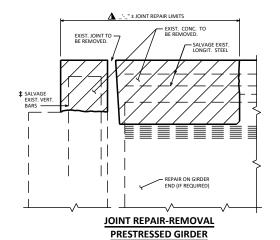
JOINT TIES ARE REQUIRED BETWERN THE LAST TWO BARREL SECTIONS ON SKEWED SKEWED STRUCTURES. WHEN JOINT TIES ARE REQUIRED IN OTHER LOCATIONS, AS DETERMINED BY THE ENGINEER, PROVIDE A PLAN NOTE OR LIMITS IDENTIFYING REQUIRED JOINT TIE COCATIONS. SITES SUSCEPTIBLE TO DIFFERENTIAL SETTLEMENT MAY WARRANT THE REQUIREMENT OF JOINT TIES THROUGHOUT THE BOX CULVERT LENGTH.

# PRECAST CONCRETE BOX CULVERT BARREL DETAILS



APPROVED: Laura Shadewald





# # SALVAGE EXIST. DINT TO BE REMOVED. # SALVAGE EXIST. LONGIT. STEEL # SALVAGE EXIST. PLATE AND ANGURES CONSTRUCT HEND DEFAIL "STEEL GROER WITH END DIAPHRAGM". # SALVAGE # S

## LEGEND

- EXISTING BARS ARE LIKELY TO BE CORRODED AND/OR DAMAGED DURING CONCRETE REMOVAL SALVAGE AND INCORPORATE AS MUCH REBAR AS PRACTICAL. SUPPLEMENT WITH THE BARS INDICATED BY
- ADHESIVE ANCHORS NO. 5 BAR. EMBED 1'-0" IN CONCRETE. SPACE AT 1'-0".
  TURN 10" LEG AS NECESSARY TO FIT.
- OPT. CONST. JT. 1" MIN. BELOW EXIST. REINF.

#### **DESIGNER NOTES**

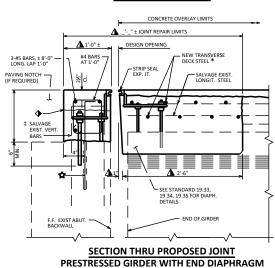
SEE STANDARD 28.01 FOR SUPPORTS USED FOR STRIP SEAL STEEL EXTRUSIONS.

\* FOR SKEWS > 20°, WHERE ORIGINAL TRANSVERSE DECK REINFORCEMENT WAS PLACED NORMAL TO THE GIRDERS, SAVE AND INCORPORATE 1'-6" MIN. OF TRANSVERSE REINFORCING BARS. NEW TRANSVERSE BARS ARE PLACED ALONG THE

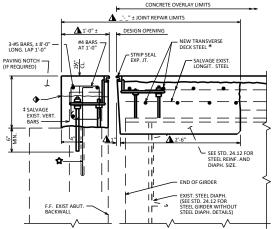
BARS IN JOINT REPAIR SHALL MATCH EXISTING REINFORCEMENT TYPE (COATED OR UNCOATED).

ALL REPLACEMENT PAVING BLOCK DIMENSIONS SHALL MATCH EXISTING PLAN DIMENSIONS UNLESS DESIGNER DETERMINES OTHERWISE, TYP. FOR ALL SECTIONS SHOWN ON THIS STANDARD.

FOR STEEL GIRDERS, USE BID ITEM "PREPARATION AND COATING OF TOP FLANGES (STRUCTURE)" FOR JOINT REPAIRS OR DECK REPLACEMENTS.

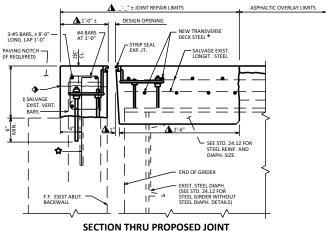


CONCRETE OVERLAY



SECTION THRU PROPOSED JOINT
STEEL GIRDER WITH END DIAPHRAGM
CONCRETE OVERLAY

5



STEEL GIRDER WITH END DIAPHRAGM
ASPHALTIC OVERLAY

#### **TOTAL ESTIMATED QUANTITIES**

BID ITEM NUMBER	BID ITEMS	UNIT	TOTAL
502.3101	EXPANSION DEVICE	LF	
502.4205	ADHESIVE ANCHORS NO. 5 BAR	EACH	
509.1000	JOINT REPAIR	LF	
509.2100.5	CONCRETE MASONRY DECK REPAIR	CY	
	POSSIBLE ADDITIONAL BID ITEMS		
505.0400	BAR STEEL REINFORCEMENT HS STRUCTURES	LB	
505.0600	BAR STEEL REINFORCEMENT HS COATED STRUCTURES	LB	
509.2500	CONCRETE MASONRY OVERLAY DECKS	CY	
517.0901.S	PREPARATION AND COATING OF TOP FLANGES (STRUCTURE)	EACH	

STRIP SEALS & DIAPH.
DETAILS FOR OVERLAYS



APPROVED: Laura Shadewald